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1 **The Role of Recycled Rubber Inclusions on Increased Confinement in Track Substructure**

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26 **Technical Note**, Submitted to Transportation Geotechnics

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37 **The Role of Recycled Rubber Inclusions on Increased Confinement in Track Substructure**

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54 **Abstract:** Large cyclic and impact loads exerted by heavy haul trains can cause significant
55 deformation and degradation of ballast, leading to poor track geometry and track instability. The
56 application of recycled rubber elements in track substructure to increase confinement of both sub-
57 ballast and shoulder ballast is an innovative solution. In Australia, there is a lack of adequate recycling
58 that leads to large stockpiles of waste tyres. In addition, the reusability of giant off-the-road tyres
59 discarded from mining industry is seriously limited due to their size and weight (over 3.0 meters in
60 diameter weighing about 3 tonnes). This study presents a real-size prototype test using the Australia's
61 first and only National Facility for Cyclic Testing of High-speed Rail to investigate the performance
62 of a hybrid track where tyre-infilled granular waste materials were placed below the ballast layer to
63 replace the traditional capping layer, and arc segments cut from the giant off-the-road tyres were used
64 to confine shoulder ballast. The performance of this hybrid track is compared with an unreinforced
65 track conducted earlier at the same loading conditions. Test results demonstrate that the use of this
66 hybrid system with recycled rubber elements significantly decreases vertical and lateral
67 displacements of ballast and effectively controls the distribution of vertical stress with depth, while
68 reducing vibration and ballast breakage. The outcomes of this study provide a unique solution in a
69 circular economy perspective to strengthen railways to cater for heavier and faster freight trains.

70 *Keywords: Ballast, rubber tyres, confining pressure, displacement, particle breakage*

71

72 **1. Introduction**

73 Heavier and faster rail corridors are of prime importance in transportation infrastructure development
74 and require insightful studies to improve the future design of railways under increasing axle loads
75 and speeds [1-3]. However, increasing speed, tonnage, and heavier axle load can present various
76 challenges related to track stability and maintenance [4-6]. Upon repeated train loading, ballast
77 aggregates become degraded and subsequently fouled by finer particles, leading to undesirable
78 performance such as increased settlements, decreased shear strength, and lower porosity that can
79 impede the drainage of the ballast layer [7-9]. Previous studies have shown that the degradation of
80 ballast seriously hampers the safety and efficiency of railways, which leads to enforced speed
81 restrictions and more frequent maintenance [6, 10, 11]. Efforts have been made lately to introduce
82 novel materials and methodologies to prevent ballast degradation and for enhanced longevity of tracks
83 with polymeric reinforcements, including geogrids to curtail lateral movement of particles and
84 geocells to provide increased confinement [12-16]. However, owing to the proximity to the sleepers
85 and the challenges during track maintenance, it is often difficult to stabilise the ballast layer with
86 conventional methods. Several researchers have studied the effect of confining pressure on granular
87 materials [17-19], and they have verified that the in-situ confining pressure plays a vital role in
88 reducing ballast breakage. As typical confinement in a ballast track is usually between 10-20 kPa as
89 measured during field studies carried out earlier by Indraratna et al. [6], there is a need for innovative
90 ways to increase the in-situ lateral confinement in tracks towards an optimum value (40-60 kPa) based
91 on extensive large-scale laboratory testing by Lackenby et al. [20].

92 With the increasing number of road vehicles, 1.4 billion tyres are sold around the world each year
93 [21]. Most of these tyres are not recycled in large quantities, hence they often end up in stockpiles
94 [22]. In some countries, waste tyres are burnt for the convenience of disposal causing severe
95 environmental pollution or they pose a potential source of spreading diseases through breeding of
96 mosquitoes during wet periods [23, 24]. Giant off-the-road (GOTR) tyres (about 3m in diameter and
97 weighing over 3 tonnes) discarded mainly from mining industry are more challenging to handle and
98 transform into rubber crumbs or mats. This study introduces the combination of two concepts of: (i)
99 placing infilled tyre assembly beneath the bottom ballast and (ii) installing GOTR tyre (arc segments)
100 in shoulder ballast to stabilise the track substructure.

101 In recent times, Indraratna et al. [25] conducted plate load tests on a single infilled tyre cell, and the
102 experimental results showed that the infilled tyre underlying the ballast layer significantly increased
103 the modulus and ultimate bearing capacity of the overall granular substructure. Other studies (e.g.,

104 Indraratna et al. [26]; Sun et al. [27] have evaluated the effects of tyre reinforcement on the ballast
105 degradation and deformation subjected to cyclic loading. These studies confirm that the inclusion of
106 recycled tyres as a capping stratum can provide substantial lateral confinement, thus enhancing the
107 lateral track stability while reducing ballast degradation.

108 To the knowledge of the authors so far, there are no other studies on the use of recycled rubber to
109 increase the confining pressure within the main load-bearing substructure of the track, including its
110 shoulders. Indeed, this is the key reason that had motivated this study at the outset. The research
111 objectives are to quantify the performance of a hybrid track. In this respect, tyre-infilled granular
112 waste materials were placed as a capping layer below the load-bearing ballast, and the arc segments
113 cut from large off-the-road tyres were used to confine the shoulder ballast. In addition, as typical
114 confinement in a ballast track is usually very low (10-20 kPa), there is a need for innovative ways to
115 increase the in-situ lateral confinement in tracks to reduce ballast breakage and control lateral
116 displacement (dilation).

117 **2. Effect of confining pressure on the performance of railway track**

118 In order to provide adequate lateral stability, the ballast layer can be extended laterally to a desired
119 width beyond the edge of sleepers [1]. However, the in-situ increase in confining pressure at the track
120 shoulders is still relatively small [20]. Under repeated cyclic loading, this low confining pressure is
121 not sufficient to effectively control excessive lateral spreading of aggregates. Indraratna et al. [6] have
122 shown that optimising the track confinement can contribute to substantial savings on track
123 maintenance. The role of confining pressure on shear strength and deformation behaviour of rockfill
124 has been extensively studied earlier. For instance, Marsal [28] reported that the shear strength of
125 basaltic and granitic aggregates could be modelled by a non-linear function of the normal stress.
126 However, the confining pressure in these tests has been in the high range of 0.5 to 2.5 (MPa), which
127 is most unlikely to be encountered in a ballast track. Indraratna et al. [29] presented more realistic
128 results by varying the confining pressure in a much lower range (1 kPa to 240 kPa) to conclude that
129 the internal friction angle and dilation of the granular mass are significantly governed by the confining
130 pressure, as also supported by others (e.g., [19, 30]). In real-life field situations, the track confining
131 pressure is often very low and typically less than 30 kPa [31] and highly angular ballast aggregates
132 experience significant dilation and associated breakage under cyclic loading. Lackenby et al. [20]
133 found that under higher confining pressure, dilation of ballast is suppressed, and the degradation
134 mechanism also changes from the breakage of angular corners to splitting across the body of particles.
135 In this context, Indraratna et al. [17] proposed three different zones of particle degradation in relation

136 to confining pressure, namely, (a) dilatant, unstable degradation zone (DUDZ, $\sigma_3 < 30$ kPa), (b)
137 optimum degradation zone (ODZ, $30 < \sigma_3 < 75$ kPa) and (c) compressive, stable degradation zone
138 (CSDZ, $\sigma_3 > 75$ kPa).

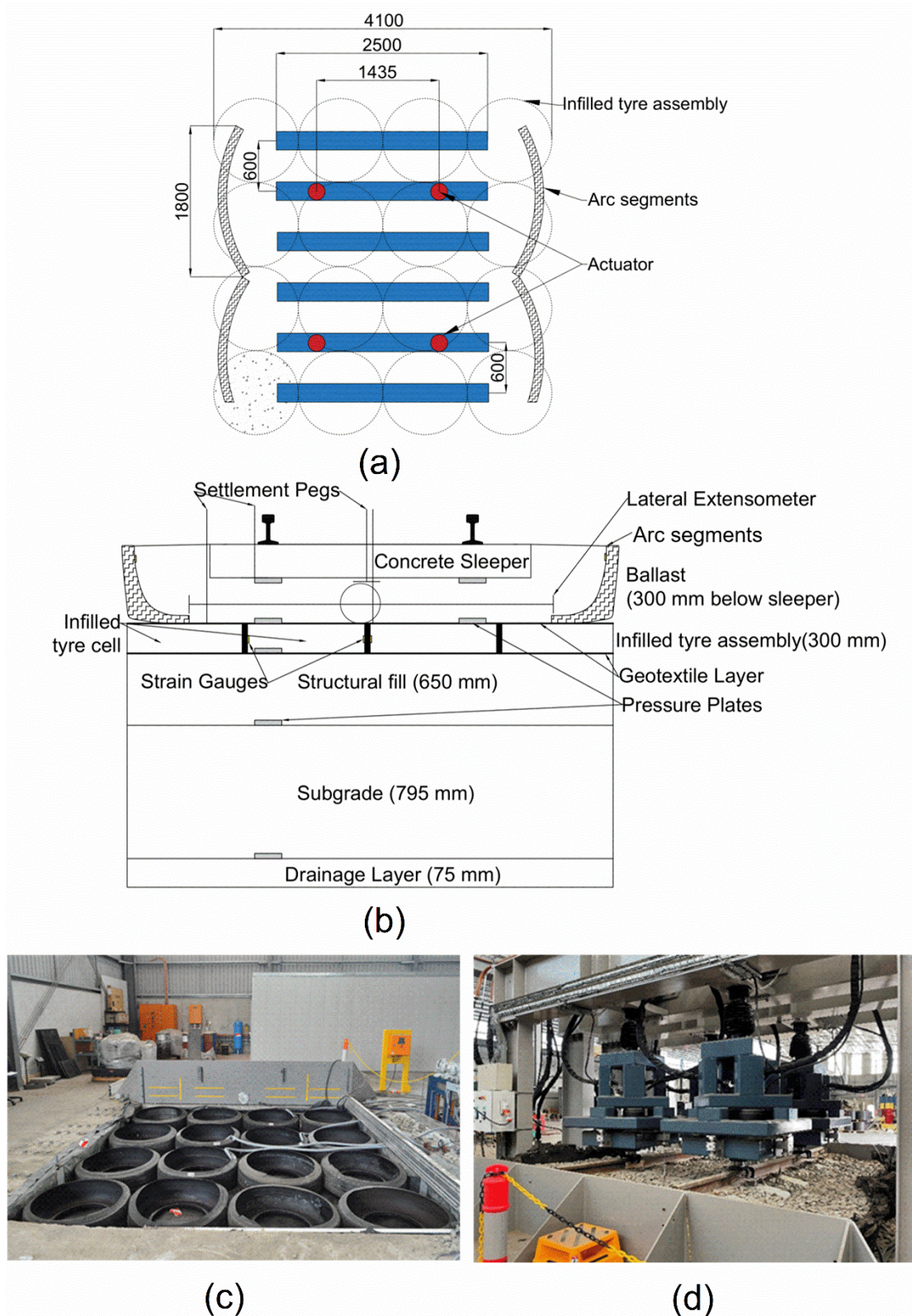
139 **3. Application of Recycled Rubber Elements to Construct a Hybrid Track**

140 Although theories and conceptual developments of internal confinement attributed to frictional forces
141 and hoop stress for confined granular materials [32, 33], and energy absorption mechanisms of rubber
142 inclusions [34] are still evolving, there is limited scientific evidence and field verification on the use
143 of recycled tyres in rail tracks. In addition, the concept of employing rubber tyres for improved track
144 performance considering a 3D cylindrical geometry has not yet been examined in practice. Several
145 recent attempts were based on static loading [25] or cyclic loading with constrained boundary
146 conditions replicating a unit cell conceptual model [26].

147 To mitigate problems related to the lack of ballast confinement, one solution offered by the authors
148 is to use arc segments of the giant off-the-road (GOTR) tyres to provide additional support to the
149 shoulder ballast, where each cut tyre arc segment may have a central subtended angle between 40°
150 and 60° , having a weight of around 170 to 260 kg. The entire tyre assembly placed in the track
151 substructure is shown in Fig. 1. In addition to this self-weight of the arc segments, the weight of
152 aggregates confined by the tyre segments also acts as a passive stabilising force against the lateral
153 displacement of the crib and bottom ballast. This resulted in two GOTR tyre segments around 1800
154 mm in length to be placed in the longitudinal direction of the track on each side of the track, and the
155 tyre segments were instrumented with strain gauges. Subsequently, the shoulder ballast was placed
156 and compacted within these arc segment boundaries to increase the overall track confinement (Fig.
157 1a). The confining arc segments were placed next to each other without any rigid connection so that
158 they can independently displace in the horizontal direction (simulating lateral displacement of tracks).
159 A schematic cross-section of the test pit is constructed with layers of different materials and depths
160 as shown in Fig. 1b. These rubber tyres are produced to take high heat and abrasion and to resist
161 degradation under continual wet conditions. Tyre supplies companies have done much research to
162 prove that any breakdown of this heavy-duty rubber when not exposed to UV light will take over 150
163 years before any significant degradation to occur (Goryunov et al. [38]; Bridgestone [39]). It is also
164 noted that in the very long term (> 100 years), rubber tyres may very gradually degrade and leach out
165 into the environment.

166 **4. National Facility for Heavy-haul Railroad Testing**

167 National Facility for Cyclic Testing of High-speed Rail (FCTHSR) is the first of its kind in Australia
168 to be conceptually designed by the 1st author to accommodate a wide array of track conditions. It is
169 expected to provide a national and international hub for high-quality collaborative research with the
170 main objective of contributing to better and more cost-effective track design solutions catering for
171 faster freight trains carrying heavier loads. More elaborate details of the conceptual design,
172 construction process and technical specifications for testing are discussed elsewhere by Indraratna et
173 al. [35]. The first benchmark test without any rubber tyre segments (unreinforced standard track) was
174 carried out under a 25-tonne axle load and a frequency of 15 Hz subjected to a maximum of 500,000
175 load cycles. Results of settlement and lateral deformation, induced stresses, rail and sleeper
176 accelerations, and ballast breakage were reported and compared with actual field data obtained for
177 heavy haul tracks in the town of Bulli in the state of NSW and, with the large-scale physical model
178 tests conducted in a process simulation apparatus. The relevant test results from these earlier studies
179 are adopted here to compare with the performance of the tyre stabilised (hybrid) track.



180

181 **Fig. 1.** The test rig and test sample assembly: (a) Top view of the test tracks; (b) cross-section of the

182 test track; (c) a photo of recycled tyre assembly; (d) photo of a complete track with GOTR arc

183 segments

184 *4.1. Test setup for the hybrid track with the recycled rubber elements*

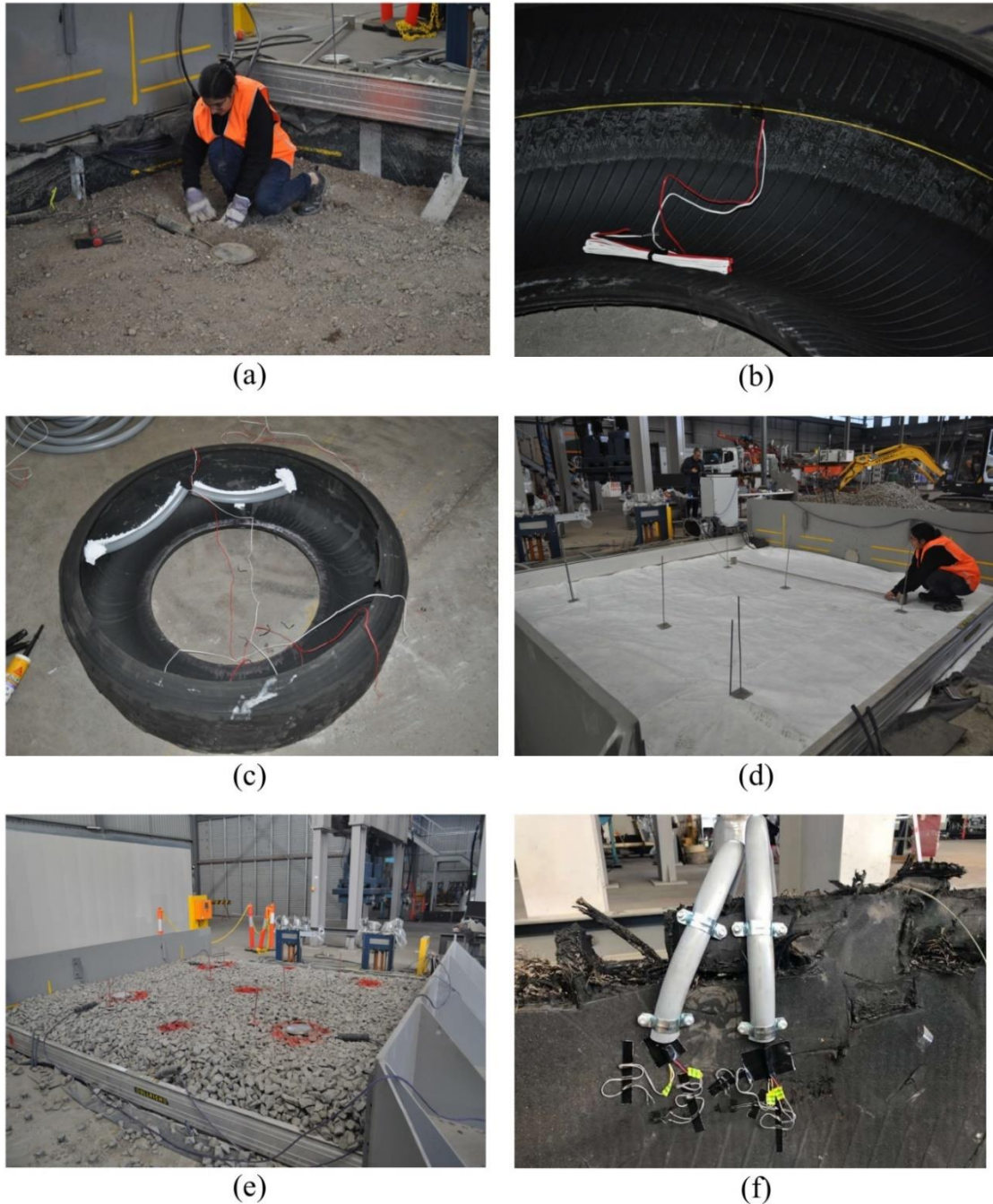
185 The FCTHSR facility was used to investigate the performance of the hybrid track reinforced with: (i)
186 recycled rubber tyre cells as capping (tyre cell assembly, Fig. 1c); and (ii) GOTR arc segments
187 installed in the shoulder ballast (Fig. 1d). A drainage layer (75mm thick) consisting of coarse-grained
188 gravel was placed at the bottom of the test pit, overlain by a 795 mm-thick layer of fine-grained soil
189 (subgrade) compacted to a unit weight of 16.5 kN/m^3 , corresponding to 95% degree of compaction
190 based on Standard Proctor recommendations. In the conventional track test, a layer of geotextile was
191 placed on top of a 75mm thick drainage layer to prevent pore pressure build-up, and this condition
192 was kept the same for the hybrid track test. A 650mm-thick structural fill layer was then placed on
193 the subgrade and compacted to a unit weight of 18.5 kN/m^3 . Instead of the traditional capping layer,
194 the layer of recycled-rubber tyre cells was assembled and infilled with granular waste and compacted
195 with a vibratory plate to a unit weight of 19.5 kN/m^3 . In addition, a hand-held rod vibrator was used
196 to compact around the tyres to eliminate any gaps. In total, 16 truck tyre cells (1m in diameter and
197 275~300 mm in height) were used. It is noteworthy that this assembly of tyre-infilled granular waste
198 not only provide significant lateral confinement while increasing the track stiffness, but also possess
199 energy absorbing properties to alleviate undue noise and vibrations. The tyre cell assembly was then
200 overlain by fresh angular ballast (i.e., latite basalt of volcanic origin) of 300 mm thickness,
201 conforming to Australian Standards [36]. The ballast stratum was compacted in three sub-layers (each
202 100mm-thick) to achieve a unit weight of approaching 16.0 kN/m^3 as usually expected in Australian
203 heavy haul tracks. The particle size distribution (PSD) curves for ballast, subgrade, and structural fill
204 layers have been described in detail by the authors in a previous study [35]. It is noted that different
205 gradation curves of the ballast layer would affect the measured vibration and ballast breakage in
206 hybrid tracks. For instance, the coarser ballast fractions would result in higher breakage and increased
207 vibration. However, a study of the effect of different gradation curves of ballast is beyond the scope
208 of this study.

209 The thickness of ballast layer carried out in the tests was 300 mm, followed by a 300mm-thick waste
210 granular infill compacted within the tyre cell assembly sandwiched between the overlying standard
211 ballast and the underlying structural fill and subgrade media. The infill material within the 300mm
212 thick rubber tyre cells is a granular medium which can be taken from most waste piles in railway
213 yards. Mixtures of capping material, drainage material and spent/broken ballast often constitute these
214 waste granulates. There is no apparent shakedown condition that we can comfortably establish based
215 on the observed data, and the continual settlement is also was influenced by the deformation of the
216 infilled material within the tyre cell assembly, which now substitutes the traditional capping layer

217 that is usually composed of quarried materials (albeit much smaller in particle sizes compared to
218 standard load-bearing ballast). The idea of a rubber tyre cell assembly is to provide much better
219 confinement plus energy absorbing (damping) capability to this substituted capping layer to alleviate
220 impact damage from track irregularities. In practice, a drainage layer and a geotextile layer are
221 commonly placed underneath the infilled-granular tyres (e.g., Chullora field testing site) so that water
222 can be drained out and related issues with water trapping within the tyres can then be eliminated.

223 **4.2. Instrumentation and loading program**

224 The hybrid track consisting of the central tyre cell assembly plus the shoulder tyre arc segments was
225 instrumented with various sensors to monitor the track response during the application of cyclic
226 loading. An array of instrumentation (pressure cells, Linear Variable Differential Transformers,
227 moisture sensors, pore pressure transducers, accelerometers and strain gauges) were placed in
228 specified strategic locations. All these measuring devices have been individually tested and calibrated
229 prior to installation in the FCTHSR. Stresses at different interfaces were recorded by pressure cells,
230 as shown in Fig. 2a. Pressure cells placed at the bottom layer of tyre-infilled granular waste material
231 that had replaced the traditional capping layer in the sub-ballast. Four biaxial strain gauges (Fig. 2b)
232 were installed inside the tyre cell wall and protected by Permatex medium-strength glue (Fig. 2c) to
233 measure the vertical and circumferential lateral strains. Four lateral extensometers were placed in the
234 ballast layer to measure the lateral displacement. Sixteen settlement pegs were installed at different
235 depths (infilled tyre assembly, ballast and underneath sleepers) to measure the settlements of this
236 hybrid track (Fig. 2d). Pressure cells were also placed at the top and bottom of ballast layer to measure
237 the vertical and lateral stresses at various locations (Fig. 2e). Four biaxial strain gauges (Fig. 2f) were
238 also installed in the GOTR arc segments to measure the relevant strains during testing. The
239 accelerometers were used to determine the accelerations of rail and sleepers.



240

241 **Fig. 2.** Instrumentation arrangement in the test rig: (a) pressure cells placed at the bottom layer of
 242 tyre-infilled granular waste material; (b)&(c) strain gauges installed in tyres; (d) installation of
 243 settlement pegs and lateral extensometers in ballast; (e) pressure cells in ballast; (f) closed-view of
 244 strain gauges installed in a GOTR arc segment

245 The test was carried out under sinusoidal cyclic loading characteristics with maximum load
 246 (P_{max})=125 kN, minimum load (P_{min})=15 kN and mean load (P_{mean})=70 kN, simulating an equivalent
 247 25-tonne axle freight train. The applied frequency of $f=15$ Hz represents a realistic range of heavy
 248 haul train speeds of 50-60km/h based on Australian standard gauge tracks. The selected sample rate

249 (data logging frequency) for all the signals was 1200 Hz. According to USACE [40] railroad design
250 manual assumes that the wheel point load is distributed between five sleepers, with the maximum
251 load directly beneath the wheel (40% of the total load). The system of concrete sleepers and steel rails
252 are connected to each other via rail join fastening, including clips and rail pads provided by the
253 Australian Rail Track Corporation (ARTC) that are commonly used in NSW, Australia. It is the same
254 as the components used in benchmark testing [35]. Steel rails (typed: AS60) were used in the test
255 (density 7850 kg/m³, elastic modulus: 210,000 MPa). Hyperelastic materials were used as the rail
256 pads having an equivalent vertical stiffness of 200 kN/mm. It is also noted that the initial conditions
257 of ballast, subgrade and structural fills are the same as those carried out in benchmark testing [35]
258 for the results to be compared. The test was completed after 500,000 loading cycles, and during the
259 application of the cyclic load, the settlements, lateral displacements, stress distribution with depth,
260 acceleration of rail and sleepers were recorded by an automatic data acquisition system. Ballast
261 aggregates were sieved before and after the test to measure the ballast breakage index (BBI).

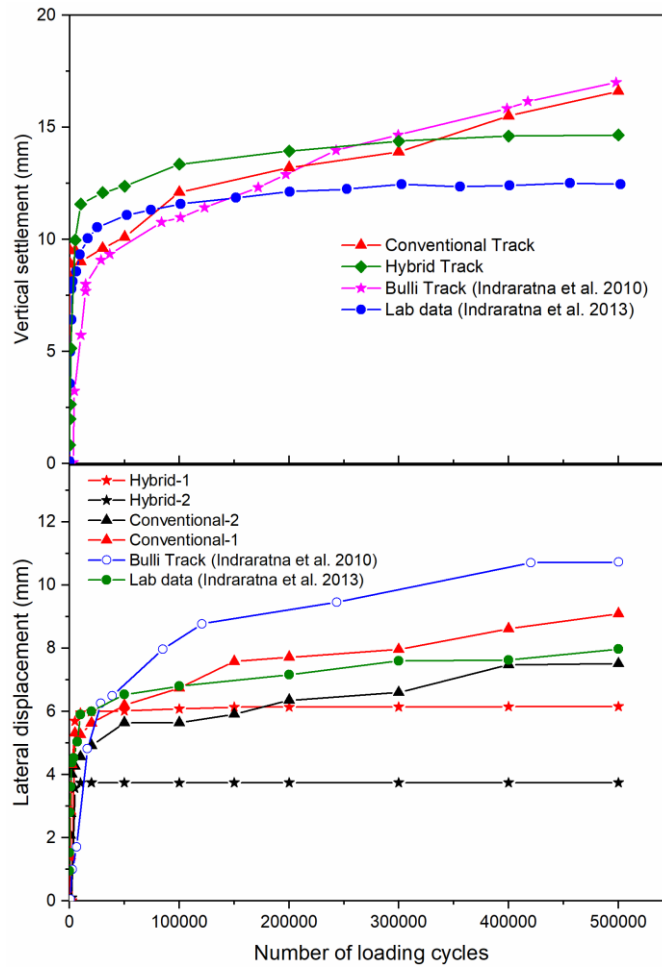
262 **5. Results and discussion**

263 *5.1. Effect of recycled rubber elements on settlement and lateral displacements*

264 The measured vertical settlement results from the hybrid track were analysed and compared with
265 those measured results reported earlier for the unreinforced FCTHSR testing (Indraratna et al. [35],
266 large-scale process simulation triaxial testing [37] and selected field data taken from Bulli heavy haul
267 track [6], as shown in Fig. 3. It is noted that the ballast depth at Bulli was also 300 mm (same as
268 conventional tracks in NSW and also as used in our hybrid track tests). A structural fill layer
269 (engineered fill layer with adequate permeability and shear strength) was also used in the Bulli field
270 testing [6]. All results show a similar initial rapid settlement up to N=100,000 cycles, followed by
271 gradually increasing settlement within N=300,000, and then remaining relatively stable to the end
272 (N=500,000). When the number of load cycles is smaller than 300,000, the vertical settlement of
273 hybrid track is larger than that of a conventional track. This is mainly due to ballast in the hybrid track
274 experiencing a pronounced re-arrangement and subsequent compression during the initial N=300,000
275 cycles to attain a more stable track. However, over a large number of loading cycles (N=500,000),
276 the overall accumulated settlement of the hybrid track is smaller than that of a conventional track. As
277 the confining arc segments were placed away 300mm from the edge of sleepers, the increased
278 confining pressure in hybrid track is found to be more effective in reducing ballast breakage [20],
279 apart from its additional role of controlling ballast deformation. The hybrid track achieves notable
280 stability around N=100,000, which certainly seems quicker than the unreinforced track. From a

281 practical perspective, this suggests that a newly constructed hybrid track is able to attain operational
282 stability earlier than a traditional ballast track, and thus, tamping would be recommended fairly early
283 in actual practice.

284 Fig. 3 also compares the measured lateral displacement recorded by lateral extensometers placed in
285 the ballast layer between the conventional and hybrid tracks. Typical data obtained from the Bulli
286 field site and from large-scale laboratory testing conducted under similar cyclic loading conditions
287 are also plotted for comparison. Lateral displacements were measured at underneath the sleepers and
288 at the bottom of the ballast layer. It is seen that the lateral displacement of the hybrid track ranges
289 from 3~6 mm with an average displacement of 4 mm, while the lateral displacement of the
290 conventional track is significantly higher at about 9 mm. There is no doubt that the shoulder
291 confinement by rubber arc segments effectively curtails the lateral movement of ballast. The system
292 of recycled rubber elements can provide more than double the lateral confining pressure of the hybrid
293 track ($\sigma_3 = 55\text{kPa}$) as measured by a pressure cell placed vertically within the shoulder ballast (Fig.
294 2e), compared to ($\sigma_3 = 23\text{kPa}$) the conventional track as reported earlier by Indraratna et al. [35].
295 These results are in sound agreement with Lackenby et al. [20] who have earlier shown using large-
296 scale process simulation triaxial testing that rail ballast tested at low confining pressures experience
297 excessive lateral displacements hence dilation, while increased confining pressure reduces dilation
298 by controlling the lateral strain.



299

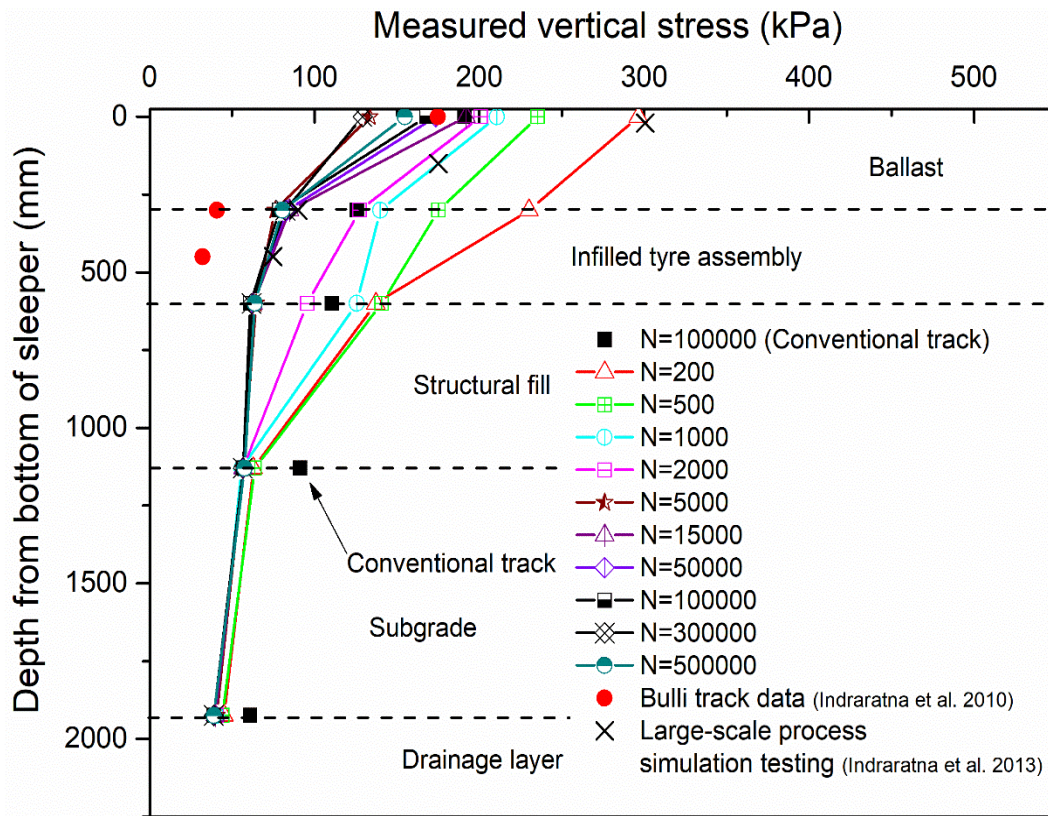
300 **Fig. 3.** Measured vertical settlements (at the top of the ballast layer) and lateral displacements of the
 301 hybrid track during the test

302 **5.2. Measured stress distribution with depth**

303 Fig. 4 presents the vertical stress (σ_v) recorded by pressure cells which were placed under the vertical
 304 alignment of the sleepers and actuators, as shown in Figure 1b. At the beginning of the loading stage
 305 ($N < 2000$ cycles), the vertical stresses measured in the hybrid track are higher than those for the
 306 conventional track. However, over a large number of loading cycles ($N > 2,000$), the vertical stresses
 307 measured in the hybrid tracks are smaller than those of a conventional track. It is seen that the stress
 308 levels at different depths reduce with an increasing number of loading cycles until $N = 100,000$ beyond
 309 which the stresses remain relatively unchanged. This agrees with the measured vertical settlements
 310 after $N = 100,000$ cycles shown earlier in Fig. 3. It is noted with interest that both the primary ballast
 311 layer and the underlying infilled tyre assembly take a large proportion of the substructure stresses,
 312 thus effectively controlling the propagation of any excessive stress to the underlying softer layer

313 including the subgrade soil, which is indeed the key purpose of a sound capping stratum beneath the
314 load-bearing primary ballast layer.

315 Compared to the conventional track [35], the hybrid track (reinforced by recycled rubber elements)
316 shows significantly decreased vertical stresses on the ballast layer, infilled tyre cell assembly
317 (capping) and the subgrade layer. For instance, at N=100,000 cycle, the vertical stresses measured at
318 the layers of ballast, tyre cell assembly and subgrade were $\sigma_v = 164, 76$ and 52 kPa, compared to $\sigma_v =$
319 $191, 126$ and 91 kPa for the conventional track. In essence, the proposed system of recycled rubber
320 elements has contributed to about 40% and 43% reduction in the vertical stress at the sub-ballast and
321 subgrade layers, respectively. It is noted that the loading distribution between the sleepers depends
322 on the rail type, rail pad stiffness, and properties of the foundation, among others. So, these parameters
323 were kept almost the same for the current tyre assembly track testing and the previous benchmark
324 track testing [35] to ensure a proper comparison between results. Fig. 4 also compares the stress
325 distribution with the field trial conducted at Bulli, NSW, with similar axle-loading [6]. It can be
326 observed that the vertical stress measured at the bottom of the ballast layer in the field trial is less
327 compared to the values obtained during the test. This difference can be attributed to having a rigid
328 boundary (reinforced concrete) at a depth of about 2m in the testing facility. In addition to that, the
329 presence of infilled tyre assembly below the ballast layer increases the average stiffness of the
330 foundation, which can lead to higher stress at that depth. It can also be noted that the values obtained
331 from the current test agree quite well with the large-scale process simulation testing conducted in the
332 laboratory which also has a rigid boundary condition [37]. The high strain energy capacity and
333 increased damping properties attributed to rubber elements within the infilled-tyre assembly are
334 clearly contributory factors in reducing the stress magnitude transferred to other substructure layers.
335 This concept is explained elsewhere [34] and not elaborated within the scope of this paper.



336
337 **Fig. 4.** Vertical stress distribution along the depth in the hybrid track compared with the
338 conventional track, field and laboratory data

339 **5.3. Measured vibration of rail and sleeper**

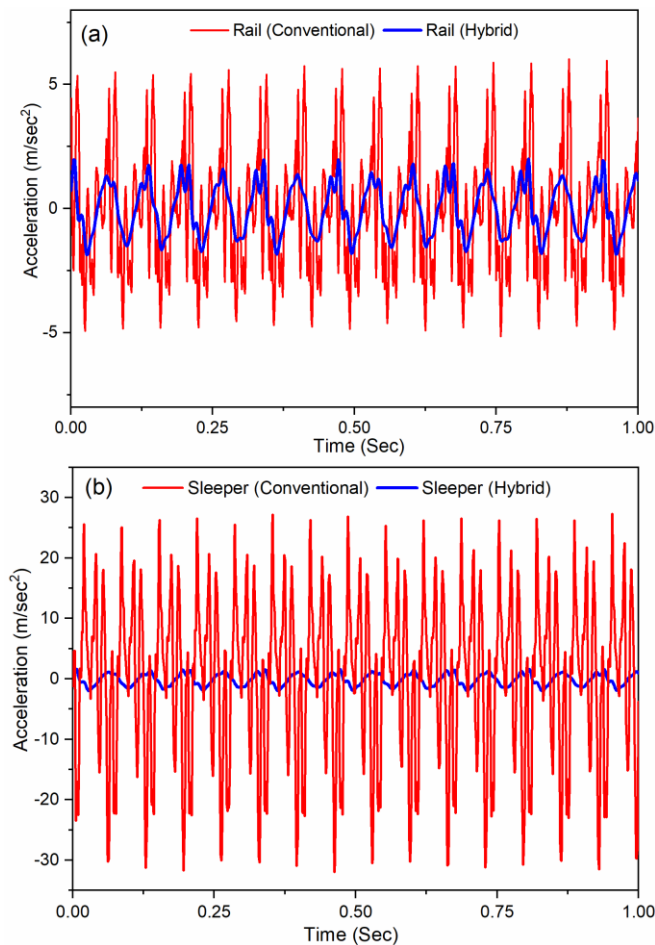
340 Track vibrations were recorded during the test by two accelerometers that were rigidly bonded on the
341 rail and sleeper assembly to measure acceleration (vibration). Fig. 5a compares accelerations
342 measured on the rail in the hybrid track with those of the conventional/unreinforced track reported by
343 Indraratna et al. [35] at N=200,000 cycles. It is seen that the accelerations in the hybrid track are less
344 than half of the conventional track. In fact, the maximum accelerations measured on the rail of the
345 hybrid track (A_{R_hybrid}) and the conventional track (A_{R_con}) are 2.47 m/s^2 and 5.60 m/s^2 , respectively.
346 It is noted that the dynamic response of track is influenced by the test setup and loading and boundary
347 conditions. The measured accelerations reveal that for the given test setup and testing conditions
348 carried out in this study, the inclusion of recycled rubber elements in the track substructure can
349 significantly reduce vibration.

350 Fig. 5b shows a comparison of the measured accelerations on a sleeper of the hybrid track with those
351 on the conventional track at N=200,000 cycles. While the acceleration measured for the hybrid track
352 (A_{S_hybrid}) is only about 2.07 m/s^2 , the recorded acceleration for the conventional track (A_{S_con}) was
353 more than ten times greater at 27.5 m/s^2 . There is no doubt that from a practical perspective, the

354 hybrid track can experience much less vibration based on these data. It can also be observed that the
355 degree of vibration reduction is much more in the sleeper than in the rail because of the proximity to
356 the infilled tyre assembly which acts as an energy-absorbing stratum. The acceleration signals on the
357 rail and sleepers from the benchmark and hybrid tests do closely resemble the sinusoidal wave of the
358 cyclic loading, as clearly denoted by 15 loading cycles per second, as expected.

359 It is noteworthy that accelerometers used by the authors to measure accelerations in both cases of
360 track testing (i.e., conventional and hybrid) are indeed the same. The type of glue, position of
361 measuring points, the applied axle load frequency (15 Hz), the measurement systems and the
362 corresponding data logging frequency are the same for both cases for the purpose of direct
363 comparison. During testing of the conventional track, Indraratna et al. [35] observed undue vibration
364 on a hanging sleeper attributed to excessive lateral displacement of ballast within the close proximity
365 to the sleeper. The resulting gap between the sleeper and the ballast layer had contributed to
366 significantly increased acceleration measured on this hanging sleeper. Given that the sleeper-rail
367 assembly was firmly installed within the well-compacted ballast layer, any re-adjustment of the
368 ballast in the sleeper vicinity to seal the gap was not practical without disturbing the surrounding track
369 substructure. In addition, the authors wish to clarify that the conventional test [35] was subjected to
370 the same loading frequency of 15 Hz as the hybrid test presented in the current study. The
371 measurements were taken exactly at the same positions in the substructure. Also, the same signal
372 processing was applied (low pass filtering), adequate sampling frequency was considered, and the
373 results corresponded to equivalent loading cycles, i.e. $N=200,000$.

374



375

376 **Fig. 5.** Measured accelerations on top of (a) rail & (b) sleeper compared with the conventional track

377 **5.4. Confinement by rubber elements and reduction in ballast breakage**

378 Subjected to cyclic loading, the infilled tyre assembly was duly activated by sufficient vertical
 379 compression (axial strains in the proximity of 5%) and corresponding inducement of lateral
 380 circumferential strains albeit much smaller ($< 2\%$). It is noteworthy that these truck tyres along their
 381 circumference are heavily reinforced by steel wires to provide very high stiffness, hence the relatively
 382 small circumferential strains even under significant hoop stress generated during loading. In contrast,
 383 at the track shoulder the tyre arc segments (1.5m away from the track centreline) were not subjected
 384 to any vertical loading, and also their shape and sheer weight (each weighing 250 kg) imposed an
 385 ideal non-displacement boundary as a gravity wall (i.e., negligible sliding) that effectively restrains
 386 the lateral movement of shoulder ballast. During cyclic loading, the measurement of insignificant
 387 circumferential tensile strains ($< 0.03\%$) in these arc segments further implied insignificant lateral
 388 stress applied by any notable movement of shoulder ballast.

389 The compression of infilled granular mass upon loading will always induce some lateral (radial) push
 390 by the displacing aggregates against the tyre cell periphery thus effecting tensile (hoop) stress. This
 391 hoop stress is accompanied by an additional confinement for the infill materials, the magnitude of
 392 which depends on the circumferential stiffness (k_c) of the tyre cell. The study conducted by Indraratna
 393 et al. [32] on a system of geocell-reinforced sub-ballast subjected to cyclic loading, presented a plane
 394 strain model to determine the hoop stress and the corresponding additional confining stress, by
 395 assuming an equivalent circular area for the geocell pocket. For a tyre cell, Eq. 1 can be used to
 396 calculate the additional confinement based on the circumferential stiffness of the rubber tyre as
 397 follows [32]:

$$\sigma_c = \frac{k_c}{(1 + \mu) \times (1 - 2\mu)} \times [(1 - \mu)\varepsilon_c + \mu\varepsilon_3] \quad (1)$$

398 where, σ_c = circumferential stress, k_c = circumferential stiffness of tyre cell, μ = Poisson's ratio of
 399 rubber tyre, ε_c & ε_3 = circumferential and radial strains developed in the tyre.

400 Based on tensile testing, the circumferential stiffness of tyre cells varied in the range: 2500-3000
 401 kN/m, and the calculated hoop stress was 108.3-111.5 kPa, with an associated additional confinement
 402 of 21.6-22.3 kPa. In fact, this calculated confining stress was comparable to the pressure cell
 403 measurements in the range of 16-23 kPa at locations just above and within the tyre cell stratum. For
 404 latite basalt aggregates with an internal friction angle of 48°-50° determined from large-scale triaxial
 405 and direct shear testing [31], the resulting lateral earth pressure coefficient is in the range of 0.13-
 406 0.15. Considering the normal stress of 225 kPa measured from an embedded pressure cell the apparent
 407 internal confining pressure within the ballast layer could be estimated to be in the proximity of 32
 408 kPa, which is in agreement with past field data obtained from standard tracks [31]. Together with the
 409 additional confinement generated by the activated tyre cell assembly, one can reasonably anticipate
 410 the equivalent total confining pressure of this hybrid track to exceed 50 kPa. Indeed, Lackenby et al.
 411 [20] and Indraratna et al. [31] have shown with much laboratory and field evidence that the minimum
 412 ballast breakage is observed when the equivalent track confining pressure approaches at least 40 kPa,
 413 and in this regard, the tyre cell assembly has provided the desired effect, as further elaborated below.

414 After completion of 500,000 cycles of loading, ballast aggregates were collected from the track at
 415 different locations (e.g., below the sleeper & from the shoulder) in order to quantify the breakage of
 416 particles. The method proposed to quantify breakage [17] is adopted, whereby the Ballast Breakage
 417 Index (BBI) considers the area subtended by the shift in the particle size distribution (PSD) curve
 418 (before and after testing), in relation to the area between the original PSD and an arbitrary line

419 representing a crushed ballast with the smallest grain size assumed to be in the range of coarse sand
 420 starting from 2.36mm. The measured ballast breakage values are shown in [Table 1](#).

421 **Table 1.** Quantification of ballast breakage after testing for the hybrid track
 422

Sieve Size (mm)	Percentage passing		
	Initial gradation of ballast	Collected ballast underneath South-West Actuator	Collected ballast at the end of the sleeper (shoulder ballast)
63	100	100	100
53	67.55	71.73	69.46
37.5	17.22	19.42	18.54
26.5	4.08	4.41	4.29
19	2.12	2.45	2.32
13.2	1.56	1.65	1.59
9.5	0	0.54	0.39
4.75	0	0	0
2.36	0	0	0
	Measured BBI	0.087	0.043

423 For the hybrid track, it can be observed that the BBI values obtained below the actuator and at the
 424 shoulder are 0.087 and 0.043, respectively. When compared to the corresponding BBI values of 0.129
 425 and 0.074 determined earlier for the standard track [\[35\]](#), not surprisingly, the current hybrid
 426 counterpart indicates a substantial reduction in breakage of 33% below the actuator (BBI = 0.086)
 427 and 42% at the shoulder (BBI=0.043). This trend is further supported by the large-scale experimental
 428 data reported by Indraratna et al. [\[17\]](#) and Lackenby et al. [\[20\]](#) epitomising the reduction in ballast
 429 breakage when the confining pressure is increased from relatively low values often measured in the
 430 field (< 25 kPa) to the minimal particle degradation zone within the approximate range of 40-65 kPa.
 431 There is no doubt that the additional confinement created by the rubber-tyre stabilised substructure
 432 had significantly contributed to reducing ballast breakage. In a practical perspective this favourable
 433 outcome could be translated to longer service life of quarried ballast and reduced track maintenance
 434 cost.

435 **Conclusions**

436 This paper presents novel outcomes of including recycled rubber to increase the confining pressure
 437 at the main load-bearing part of track as well as its shoulders, based on 1:1 scale testing using the

438 Facility for Cyclic Testing of High-speed Rail (FCTHSR). Cyclic loading of prototype ballasted
439 tracks was carried out under a 25-tonne axle load applied at a frequency of 15 Hz up to 500,000
440 loading cycles. In contrast to a standard ballast track, the performance of this hybrid track consisting
441 of (i) rubber tyre assembly within the load-bearing substructure and (ii) tyre arc segments placed at
442 the track shoulders was assessed in terms of settlement and lateral displacement of granular strata,
443 vertical stress distribution with depth, ballast breakage and vibrations. Based on this study, the
444 following salient conclusions could be drawn:

- 445 • The overall accumulated settlement of the hybrid track over a large number of loading cycles
446 ($N = 500,000$) was smaller than the conventional track, albeit the initial rate of settlement of
447 the hybrid track that was slightly greater, clearly attributed to the increased compressibility of
448 the rubber tyre assembly. As expected, as a result of increase track confinement, the maximum
449 lateral displacement of the hybrid track was measured to be less than 6 mm (average of 5 mm),
450 while that of the conventional track was nearly 1.5 times (> 9 mm).
- 451 • Compared to a standard ballast track, the hybrid track significantly reduced the vertical stress
452 distribution, i.e., about 40% and 43% reduction in the stresses at the location of infilled tyre
453 assembly and subgrade layers, respectively. For instance, at $N=100,000$ cycle, the vertical
454 stresses of the hybrid track at the ballast, sub-ballast and subgrade layers were measured to
455 be: $\sigma_v = 164, 76$ and 52 kPa, respectively, in contrast to $\sigma_v = 191, 126$ and 91 kPa, for the same
456 strata of the standard track without any rubber inclusions.
- 457 • Measured accelerations on the rail and sleeper of the hybrid track indicated that the
458 accompanying vibrations in the hybrid track were much less than those of the standard track.
459 For instance, the maximum acceleration measured on the hybrid track rail ($A_{R_hybrid} = 2.47$
460 m/s^2) was less than half of that of the standard track ($A_R = 5.60$ m/s^2). Moreover, the
461 acceleration measured at a hybrid track sleeper ($A_{S_hybrid} = 2.07$ m/s^2) was less than 10 times
462 that of the standard track ($A_S = 27.5$ m/s^2). These observations confirm without a doubt the
463 damping effect (i.e., energy absorbing capacity) offered by the assembly of tyre cells.
- 464 • In the hybrid track, the infilled tyre assembly in tandem with the tyre arc segments at the
465 shoulder provided the optimum confining pressure to significantly reduce ballast breakage.
466 Ballast collected beneath selected sleepers and at the shoulders of the hybrid track indicated
467 reduced breakage indices of: $\text{BBI} = 0.087$ and 0.043 , respectively. These ballast breakage
468 reductions of 33 % and 42 % below the actuator and shoulder locations imply immense
469 benefits in relation to track maintenance costs and extended longevity of the hybrid track.

470 In essence, the outcomes of this study prove without a doubt that the use of recycled rubber elements
471 not only offer an attractive environmental solution for reduced quarrying and carbon emissions, but
472 also a technologically superior and sustainable track stabilisation option for extended life cycle and
473 reduced annual costs of maintenance of ballast tracks.

474 **CRedit authorship contribution statement**

475 **Buddhima Indraratna:** Conceptualisation, Methodology, Supervision, Formal analysis,
476 Investigation, Writing – original draft, Writing – review and editing. **Fatima Mehmood:** Formal
477 analysis, Methodology, Investigation, Writing - review and editing. **Soumyaranjan Mishra:** Formal
478 analysis, Investigation Writing –review and editing **Trung Ngo:** Supervision, Writing – review and
479 editing. **Cholachat Rujikiatkamjorn:** Supervision, Writing – review & editing

480 **Declaration of Competing Interest**

481 The authors declare that they have no known competing financial interests or personal relationships
482 that could have appeared to influence the work reported in this paper.

483

484 **Acknowledgements**

485 This study was carried out under the auspices of Industrial Transformation Training Centre for
486 Advanced Technologies in Rail Track Infrastructure (ITTC-Rail), c/o Australian Research Council
487 (ARC-IC170100006) and Discovery Project (ARC-DP220102862). The authors gratefully appreciate
488 the close collaborations with Ecoflex (c/o Jim Grant), Bridgestone (c/o Dr Shigeki Endo), Sydney
489 Trains, Australasian Centre for Rail Innovation (ACRI), and SMEC. The tyre segments were obtained
490 from Bridgestone Corporation through of Tyrecycle Pty. Ltd, Australia. Two patents for (a) infilled
491 tyre assembly (Application No. 2017900354: Track Foundation) and (b) GOTR shoulder segments
492 (Application No.2020239668: Track Ballast Confinement) are pending approval. The authors are also
493 thankful to the technical staff at UOW and UTS for their assistance during the experimental program
494 amidst COVID-19 restrictions.

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